## Distortion of Oil and Gas Infrastructure from Geomatics Support View

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#### Abstract

Above surface vertical circular tanks are commonly used in industries for storing crude oil, petroleum products, etc. and for storing water in public water distribution systems. Such tanks require periodic surveys to monitor long-term movements and settlements of the foundation or short-term deflections and deformation of the structures. One of the most effective geometric parameters of circular vertical tanks is determining it's out of roundness, distortion and the deformation as a result of age. To ensure the security of civil engineering structures, it is necessary to carry out periodic monitoring of the structures. To develop a reliable and cost effective monitoring system for the storage oil tanks, the deformation monitoring scheme consisted of measurements made to the monitored tank from several monitoring stations (occupied stations), which were established around the tanks. The circular cross section of the oil storage tanks were divided into several monitoring points distributed to cover the perimeter of the cross section. These monitoring points (studs) were situated at equal distances on the outer surface of the tanks and located around the tank base.

Geodetic instruments were setup at these monitoring stations (occupied stations) and observations carried out to determine the coordinates of monitoring points on the tank surface.

Keywords: monitoring, deformation, diameter, oil volume, intersection, accuracy, oil tanks.

### INTRODUCTION

The Forcados Yokri field is located in OMLs 43 and 45 in Burutu Local Government Area of Delta State of Nigeria. It is bounded approximately by the coordinates 319453mE to 335236mE and 148355mN to 141626mN with area coverage of 244.64sq. Km (24464.0 hectares). The entire Forcados Yorkri area features meandering creeks and mangrove swamp. The land terrain is covered by mangrove forest. The area has a humid tropical climate characterized by high rainfall and high temperature (Ehigiator, 2005). The Tanks at Forcados Terminal were constructed in the 70ths, therefore their structural integrity have been of major concern to both local community and environmentalists. Although API 653 remain the industry standard relative to tank inspection and maintenance, the frequency of testing and inspection can also be affected by various state and local regulations(Ehigiator, 2005).



Figure 1: Forcados and Environs

During the last decade, the world of engineering surveying has seen enormous developments in the techniques for spatial data acquisition. One of these developments has been the appearance of geodetic Total station

The tanks which were designed with floating roof plate of thickness 6.0mm were constructed in the 70s with the following properties (Ehigiator, 2005).

1. Norminal Diameter:	76.2m
2. Temperature:	58°f
3. Nominal Volume	100,000m <sup>3</sup>
4. Height	22m
5. Liquid Gravity	0.85 to 0.9
6. API 650 @ atmospheric temperature	

7. The hydrostatic pressure is 2 bars

Thickness: we have ten segments at vary thickness

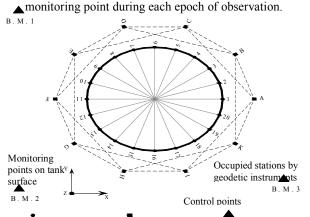
1. Bottom plate Thickness	6.0mm
2. 1 <sup>st</sup> plate thickness	34.5mm
3. 2 <sup>nd</sup> plate thickness	30.6mm
4. 3 <sup>rd</sup> plate thickness	26.7mm
5. 4 <sup>th</sup> plate thickness	22.9mm
6. 5 <sup>th</sup> plate thickness	19.0mm
7.6 <sup>th</sup> plate thickness	15.0mm
8. 7 <sup>th</sup> plate thickness	11.3mm
9.8 <sup>th</sup> plate thickness	10.0mm
10. 9 <sup>th</sup> plate thickness	10.0mm

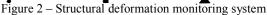
#### **Monitoring Of Vertical Storage Tanks**

It is necessary to model the structure of oil storage tank by using well-chosen discrete monitoring points located on the surface of the structure at different levels which, when situated correctly, accurately depict the characteristics of the structure.

Any movements of the monitoring point locations (and thus deformations of the structure) can be detected by maintaining the same point locations over time and by performing measurements to them at specified time intervals. This enables direct point displacement comparisons to be made. A common approach for this method is to place physical targets on each chosen discrete point to which measurements can be made. However, there are certain situations in which monitoring the deformations of a large structure using direct displacement measurements of targeted points is uneconomical. unsafe, inefficient, or simply impossible. The reasons for this limitation vary, but it may be as simple as placement of permanent target prisms on the structure is too difficult or costly (Ashraf, 2010). To obtain the correct object point displacements (and thus its deformation), the stability of the reference stations and control points must be ensured. The main conclusion from the many papers written on this topic states that every measurement made to a monitored object must be connected to stable control points (Ashraf, 2010). This is accomplished by creating a reference network of control points surrounding a particular structure (figure 2). To develop a reliable and cost effective monitoring system of any of the storage oil tanks, deformation monitoring scheme consisted of measurements made to the tanks from several monitoring stations (occupied stations), which are chosen in the area around the tank, that are referred to several reference control points (Gain, 2008). The geodetic instruments are setup at these monitoring stations (occupied stations) and observations carried out to determine the coordinates of monitoring points on the tank surface.

The circular cross section of the oil storage tank is divided into several monitoring points distributed to cover the perimeter of this cross section, as shown in figure 1. These monitoring points are situated at equal distances on the outer surface of the tank. The (stud) points are fixed, with each stud carrying an identification number and made permanent throughout the life of the tank. The purpose is to maintain the same





To determine the coordinates of occupied stations around the monitored oil storage tank, traverse network was run from the control points around the vicinity of the tank to connect the bench marks used for the monitoring.

The easiest way of visualizing the traversing process around the tank is to consider it as the formation of a polygon on the ground using standard survey procedures. The traverse was being measured using total station. The slope distances and horizontal angles were measured to survey stations on both faces for a given number of rounds, and recorded accordingly. Appropriate corrections were applied, and the distances reduced to horizontal distance. In this work, traverse network around tank N = 9 is presented (as shown in figure 3).

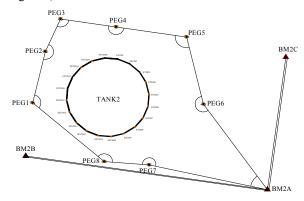


Figure 3. Traverse network for determining the coordinates of occupied stations

In fig. 3, three control points (BM2A, BM2B and BM2C) with known coordinates were fixed around the tank. Eight occupied stations (PEG1, PEG2, ..., PEG8) were established. To determine the coordinates of the eight occupied stations, a closed loop traverse was designed around tank  $N_{2}$  9 as shown in figure 3.

In this closed traverse there are 9 interior angles and 9 side lengths. The observed interior angles and sides of the traverse loop together with computed accuracy using Calson2011 software are presented in table 1.

#### **Computation and Adjustment of Observations**

By using least square theory, method of condition equation the traverse loop traverse was adjusted as follows:

The number of total observations (n) = 10 angles + 10 distances = 20.

The number of conditions (r) = 3 and these include:

1. Angular misclosure condition:

 $\Delta_1 = (\Sigma \text{ interior angle of loop traverse}) - (n_{\text{angles}} -2)(180^\circ)$  (1)

2. Sum of the departures is equal to zero (2)

$$\Delta_2 = \sum_{i=1}^n D_i * Sin \theta_i$$

Where  $D_i$  – the length of traverse side,  $\theta_i$  – bearing of traverse side

3. Sum of the latitude is equal to zero:

$$\Delta_3 = \sum_{i=1}^n D_i^* \cos \theta_i \tag{3}$$

Hence, the number of necessary observations:  $n_0 = n - r = 17$ (4)

The first step in solving traverse using conditional least square is finding the adjusted values of observations (9 interior angles and 9 lengths) and its accuracy. Secondly, from these values and accuracies, the adjusted coordinates of the traverse stations (eight occupied points) and its accuracy can be determining depending on the geometry of the traverse figure. All of these steps were carried out using Carlson2011 program. The adjusted coordinates of the traverse stations are presented in table 1.

Table 1 - Least - square solution of Tank 9 observations

Process Least-Squares Results Raw file: C:/Users/Ehigiator Raphael/AppData/Roaming/Carlson Software/Carlson2011/ICAD/SUP/TANK9.rw5 Coordinate file:

Least-Squares Closure

**Control Points** 

Point#	Easting	Northing
1	324871.298	148355.212
2	324646.398	148390.115
11	324646.398	148390.115

#### Distance Observations

Occupy	FSight	Distance	StdErr
2	3	238.670	0.035
3	4	152.605	0.035
4	5	144.176	0.035
5	6	224.836	0.035
6	7	132.289	0.035
7	8	142.874	0.035
8	9	305.188	0.035
9	10	142.232	0.035
10	11	130.610	0.035

#### Angle Observations

BSight	Occupy	FSight	Angle	StdErr
1	2	3	2°18'04"	25.227"
2	3	4	267°21'20"	39.678"
3	4	5	184°04'45"	55.715"
4	5	6	266°14'38"	42.142"
5	6	7	270°21'28"	44.368"
6	7	8	5°30'41"	42.679"
7	8	9	353°52'06"	33.839"
8	9	10	187°06'19"	43.971"
9	10	11	356°08'41"	43.044"

Adjusted Point Comparison

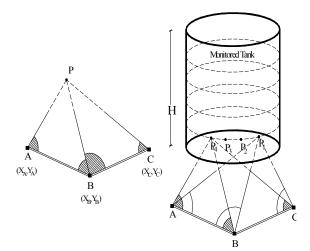
Original         Adjusted           Point#         Easting         Northing         Easting         Northing         Dist         Bearing           3         324880.585         148344.073         324880.84         148344.073         0.001         N 79°335" W           4         324880.585         148193.136         0.002         N 60°2225" W           5         324826.739         148052.409         32480.548         148193.137         0.002         N 65°2225" W           5         324604.551         148056.810         324604.548         148086.812         0.003         N 52°43'53" W           6         324604.551         148086.810         324602.5603         148217.415         0.005         N 66°05'34" W           7         324589.424         148079.197         324589.422         148077.199         0.003         N 45°3748" W           8         324589.424         148071.197         324634.728         148381.006         0.010         N 68°13'07" W           9         324634.738         148381.002         324637.707         148517.970         0.014         N 6°5'108" W
Adjusted Points Point# Easting Northing N-StdErr E-StdErr 3 324880.584 148344.073 0.002 0.002 4 324858.084 148193.137 0.003 0.003 5 324826.737 148052.410 0.003 0.006 6 324604.548 148086.812 0.005 0.006 7 324625.603 148217.415 0.004 0.004 8 324589.422 148079.199 0.004 0.006 9 324634.728 148381.006 0.003 0.002 10 324673.079 148517.970 0.002 0.004 Solution Converged in 2 Iterations Reference Standard Deviation: 0.072 Chi-Square statistic: 0.107, Range for 95%: 0.103 to 5.990 Adjustment Passed Chi-Square test at 95% confidence level
Max adjustment: 0.014
Starting Point 2: E 324646.398 N 148390.115 Z 0.000 Backsight Point 1: E 324871.298 N 148355.212 Z 0.000
Point Horizontal Zenith horz Inst Rod Easting Northing Elev No. Angle Angle Dist HT HT Description
Elev
Elev No. Angle Angle Dist HT HT Description 3 AR2.1804 90.0000 238.669 0.000 0.000 324880.584 148344.073 -0.000 PEG1A 4 AR267.2121 90.0000 152.604 0.000 0.000 324858.084 148193.137 -0.000 PEG2A 5 AR184.0445 90.0000 144.175 0.000 0.000 324826.737 148052.410 -0.000 BM-9B 6 AR266.1437 90.0000 224.837 0.000 0.000 324604.548 148086.812 -0.000 BM-9A
Elev No. Angle Angle Dist HT HT Description 3 AR2.1804 90.0000 238.669 0.000 0.000 324880.584 148344.073 -0.000 PEG1A 4 AR267.2121 90.0000 152.604 0.000 0.000 324858.084 148193.137 -0.000 PEG2A 5 AR184.0445 90.0000 144.175 0.000 0.000 324826.737 148052.410 -0.000 BM-9B 6 AR266.1437 90.0000 224.837 0.000 0.000 324604.548 148086.812 -0.000
Elev No. Angle Angle Dist HT HT Description 3 AR2.1804 90.0000 238.669 0.000 0.000 324880.584 148344.073 -0.000 PEG1A 4 AR267.2121 90.0000 152.604 0.000 0.000 324858.084 148193.137 -0.000 PEG2A 5 AR184.0445 90.0000 144.175 0.000 0.000 324826.737 148052.410 -0.000 BM-9B 6 AR266.1437 90.0000 224.837 0.000 0.000 324604.548 148086.812 -0.000 BM-9A 7 AR270.2126 90.0000 132.290 0.000 0.000 324625.603 148217.415 -0.000 BM-9C 8 AR5.3040 90.0000 142.873 0.000 0.000 324589.422 148079.199 -0.000

By the same way, the coordinates of occupied stations around each oil storage tank of ten studied tanks in the studied area in Forcados Terminal Nigeria were

determined. It is important to note that the number of monitoring points on the tank surface and the number of occupied stations around each tank differ from one tank to another depending on the topography and visibility around (Ehigiator, 2005).

# Determining the coordinates of tank surface points using linear-angular 2D intersection

To achieve accurate determination of coordinates of monitoring points on the outer surface of oil tank at Forcados terminal and its accuracy during the process of structural deformation monitoring, linear-angular intersection was used. This is because it has the advantages of least squares application. In this case, four observations were carried out from the two occupied stations (two distances and two angles). In angular intersection or linear intersection, the number of observations (two angles or two distances) equals the number of unknowns (coordinates of point P) but in case of linear-angular intersection the number of observations is more than the number of unknowns, and consequently least square method must be used to determine the coordinates of point P (figure 4). Figure (4) illustrates the geometry of the linear-angular intersection. There are two known coordinates points  $(X_A, Y_A)$  and  $(X_B, Y_B)$ . From these two known points (A and B), we can determine the coordinates of unknown point P; (X<sub>P</sub>, Y<sub>P</sub>) by measuring horizontal angles  $\alpha_1$  and  $\alpha_2$  and horizontal distances  $S_1$  and  $S_2$ . Adjustment will be carried out in this case by using observation equation method.



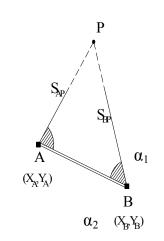


Figure 4 – Geometry of linear - angular intersection for determining point coordinates

It is important to note that the horizontal distances  $S_1$ ,  $S_2$  was measured by using reflectorless total station. Modern total station has reflectorless ability, so it can measure the inclined distance and horizontal distance without prisms.

In this model of adjustment (observational least square), the number of equations equals the number of observations (n = 4), every equation contains one observation and one or more than one unknowns. In this case, the observations are (S<sub>1</sub>, S<sub>2</sub>,  $\alpha_1$ ,  $\alpha_2$ ) and the unknowns are (X<sub>P</sub>, Y<sub>P</sub>).

The two lengths of the lines  $(S_1, S_2)$  in horizontal projection can be written in coordinates form as:

$$S_{1} = \sqrt{(X_{P} - X_{A})^{2} + (Y_{P} - Y_{A})^{2}}$$

$$S_{2} = \sqrt{(X_{P} - X_{B})^{2} + (Y_{P} - Y_{B})^{2}}$$
(5)

From figure (2), the horizontal angles ( $\alpha_1$  and  $\alpha_2$ ) can be calculated as follows:

$$\alpha_{1} = \cos^{-1}\left(\frac{AP^{2} + AB^{2} - PB^{2}}{2 \cdot AP \cdot AB}\right)$$

$$\alpha_{2} = \cos^{-1}\left(\frac{BA^{2} + BP^{2} - AP^{2}}{2 \cdot BA \cdot BP}\right)$$
(6)

By using the coordinates of points, we can write equation (2) as:

$$\alpha_{1} = \cos^{-1}\left[\frac{(X_{p} - X_{A})^{2} + (Y_{p} - Y_{A})^{2} + AB^{2} - (X_{p} - X_{B})^{2} - (Y_{p} - Y_{B})^{2}}{2 AB \sqrt{(X_{p} - X_{A})^{2} + (Y_{p} - Y_{A})^{2}}}\right]$$

$$\alpha_{2} = \cos^{-1}\left[\frac{(X_{p} - X_{B})^{2} + (Y_{p} - Y_{B})^{2} + AB^{2} - (X_{p} - X_{A})^{2} - (Y_{p} - Y_{A})^{2}}{2 AB \sqrt{(X_{p} - X_{B})^{2} + (Y_{p} - Y_{B})^{2}}}\right]$$
(7)

The equations 5 and 7 are the four observational equations, these equations are nonlinear function of both parameters and observations; they can be treated by least squares adjustment technique. The first step in the solution is finding the approximate values of unknowns. The approximated values (input data) of coordinates of point **P** (Vector  $X^0$ ) can be assumed by using angular intersection according to the following formulae (Ehigiator, 2005):

$$X_{P}^{0} = \frac{X_{A} \cot \alpha_{2} + X_{B} \cot \alpha_{1} - Y_{A} + Y_{B}}{Cot \alpha_{1} + Cot \alpha_{2}},$$

$$Y_{P}^{0} = \frac{Y_{A} \cot \alpha_{2} + Y_{B} \cot \alpha_{1} - X_{A} + X_{B}}{Cot \alpha_{1} + Cot \alpha_{2}}.$$
(8)

By substituting these approximate values in the four observation equations, the approximate values of observations  $(L^{0})$  can be computed, and then we can compute the misclosure vector (L) as follows:  $\langle \mathbf{n} \rangle$ 

$$L = L^0 - L_{obs}$$

The linearised model may be expressed in the matrix form as follows:

$$V_{(4,1)} = A_{(4,2)} \cdot X_{(2,1)} + L_{(4,1)}$$
(10)

Where A – the coefficient matrix of parameters with dimension (4, 2); L – The misclosure vector with dimension (4, 2)1); V – The residuals vector.

Matrix A can be computed by differentiation of the four equations with respect to the two unknowns and can be written in the form:

$$A_{(4,2)} = \begin{bmatrix} \frac{\partial S_{1}}{\partial X_{p}} & \frac{\partial S_{1}}{\partial Y_{p}} \\ \frac{\partial S_{2}}{\partial X_{p}} & \frac{\partial S_{2}}{\partial Y_{p}} \\ \frac{\partial \alpha_{1}}{\partial X_{p}} & \frac{\partial \alpha_{1}}{\partial Y_{p}} \\ \frac{\partial \alpha_{2}}{\partial X_{p}} & \frac{\partial \alpha_{2}}{\partial Y_{p}} \end{bmatrix}$$
(11)

By using MathCAD program, the elements  $(a_{ij})$  of the matrix (A) can be found by differentiating the four observation equations.

Then, the normal equation system is written thus:

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$$N_{(2,2)} X_{(2,1)}^{\Lambda} + U_{(2,1)} = 0$$
(12)

Where,

$$N_{(2,2)} = A^{T} \cdot W_{(4,4)} \cdot A_{(4,2)}$$
(13)

And

$$U_{(2,1)} = A^{T} W_{(4,4)} L_{(4,1)}$$
(14)

The solution for normal equation is

$$\overset{\Lambda}{X}_{(2,1)} = - N^{-1}_{(2,2)} \cdot \underbrace{U}_{(2,1)}$$
 (15)

Then, the adjusted unknown parameters can be estimated as:

$$\bar{X}_{(2,1)} = X_{(2,1)}^{\Lambda} + X_{(2,1)}^{0}$$
(16)

The vector of adjusted observations can be estimated as:

$$\bar{L}_{(4,1)} = L_{(4,1)} + \bar{V}_{(4,1)}$$
(17)

The estimated variance factor is:

$$\sigma_{0}^{2} = \frac{V^{T} . W . V}{r} = \frac{V^{T} . W . V}{2}$$
(18)

The estimated variance covariance matrix of parameters is:

$$C_X = \sigma_0^2 \cdot N^{-1} \tag{19}$$

Finally, the variance covariance matrix of the adjusted observations can be computed as:

$$C_{L} = A \cdot C_{X} \cdot A^{T}$$
<sup>(20)</sup>

By using MathCAD program, the above normal equation can be solved.

The error in point position  $M_P$  can then be determined by using the following formula (Allen, 1988):

$$M_{P} = \frac{b \ m_{\alpha}^{"}}{\rho^{"} \sin \gamma_{1}} \sqrt{\sin \frac{2}{\alpha_{1}} + \sin \frac{2}{\beta_{1}}}, \qquad (21)$$

Where b – base line (the distance between occupied stations) (for example b=AB in fig. 2);  $m'_{\alpha}$  – mean square error of measuring horizontal angles (taken from specifications of applied instrument);  $\rho'' = 206265''$ ,  $\gamma_1$  - the horizontal angle at p.

In order to accept the observations and adjusted coordinates of point P from the two triangles ABP and BCP, it is necessary that the coordinates must satisfy the following condition (Ashraf, 2010).

$$r_{P} = \sqrt{\Delta_{X}^{2} + \Delta_{Y}^{2}} \le 3 M_{t}, \qquad (22)$$
  
Where  $\Delta_{X} = X_{1}^{P} - X_{2}^{P}; \Delta_{Y} = Y_{1}^{P} - Y_{2}^{P} \text{ and } M_{t} = \sqrt{M_{1}^{2} + M_{2}^{2}}.$ 

 $X_1^{P}$ ,  $Y_1^{P}$  - Coordinates of point P from first triangle (ABP);  $X_2^{P}$ ,  $Y_2^{P}$  - Coordinates from second triangle (BCP);  $M_1$ ,  $M_2$  – Error in point position for the first and second triangles respectively (Ashraf 2010).

If the coordinates satisfy condition (22), the corrected coordinates of point P can be determined by the arithmetic mean of two triangles.

$$X_{P} = \frac{X_{1}^{P} + X_{2}^{P}}{2}, \qquad Y_{P} = \frac{Y_{1}^{P} + Y_{2}^{P}}{2}$$
(23)

The accuracy of coordinates of monitoring point P can then be determined using least square method, consider the following procedure

#### Determining the radius of the tank and its associated distortion

From coordinates of several points (more than three points) on the circumference of circular section, the radius of the tank r and coordinates of center (X<sub>C</sub>, Y<sub>C</sub>) can be determined using least square method as following:

For any monitoring point on circular section of tank surface (X<sub>i</sub>, Y<sub>i</sub>) must be fulfilled the equation of circle (Allan, 1988):

$$(\hat{x}_i - \hat{x}_c)^2 + (\hat{y}_i - \hat{y}_c)^2 - \hat{r}^2 = 0$$
  
i= 1, 2, 3, ..., n  
(24)

Where: X<sub>C</sub>, Y<sub>C</sub> - the coordinates of center of circular section, r - the corrected value of radius.

The general form of least square as following:

$$\underset{(n,u)}{A} \cdot \underset{(u,1)}{X} + \underset{(n,m)}{B} \cdot \underset{(m,1)}{V} + \underset{(n,1)}{L} = \underset{(n,1)}{0}$$
(25)  
Where:

n - The number of equations (in this case equals the number of monitoring points because each equation has one monitoring point);

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$$A_{(n,3)} = \begin{bmatrix} \frac{\partial f_1}{\partial X_C} & \frac{\partial f_1}{\partial Y_C} & \frac{\partial f_1}{\partial r} \\ \frac{\partial f_2}{\partial X_C} & \frac{\partial f_2}{\partial Y_C} & \frac{\partial f_2}{\partial r} \\ \dots & & \\ \frac{\partial f_n}{\partial X_C} & \frac{\partial f_n}{\partial Y_C} & \frac{\partial f_n}{\partial r} \end{bmatrix} = \begin{bmatrix} -2(X_1 - X_C^0) & -2(Y_1 - Y_C^0) & -2r \\ -2(X_2 - X_C^0) & -2(Y_2 - Y_C^0) & -2r \\ \dots & & \\ -2(X_n - X_C^0) & -2(Y_n - Y_C^0) & -2r \end{bmatrix}$$
(27a)

Vector V - is the vector of residual of observations

In this model the weight matrix W will have the dimension (m, m) or in other words has dimensions (2n, 2n) and has the form:

u - The number of unknowns (in this case equals 3; radius r and coordinates of center  $X_C$ ,  $Y_C$ );

m - The number of observations (in this case m = 2 n; because each point has two coordinates X, Y).

By applying least square theory, approximates values of unknowns (radius  $r^0$  and coordinates of center  $X^0$ ,  $Y^0$ ) must be assumed or calculated. To achieve this goal

The coordinates of center can be approximated by the arithmetic mean of coordinates. As following

$$X_{C}^{0} = \frac{\sum_{i=1}^{n} X_{i}}{n}, \qquad (26)$$

Approximate value of radius r<sup>0</sup> can be obtained from tank manual or by using three points on the perimeter of tank to estimate it.

The matrices can be formed by the following methods:

$$L_{(n,1)} = \begin{bmatrix} (X_1 - X_C^0)^2 + (Y_1 - Y_C^0)^2 - (r^0)^2 \\ (X_1 - X_C^0)^2 + (Y_1 - Y_C^0)^2 - (r^0)^2 \\ \dots \\ \dots \\ (X_1 - X_C^0)^2 + (Y_1 - Y_C^0)^2 - (r^0)^2 \end{bmatrix}$$
(27b)

$$\frac{\partial f_1}{\partial x_1} = 2(x_1 - x_c^0)$$
  
$$\frac{\partial f_1}{\partial y_1} = 2(y_1 - y_c^0)$$

Then, a of least squares solution is performed to find the corrected values of radius, coordinates of center, their accuracy and the distortion of the tank shell

## **RESULTS AND DISCUSSIONS**

By using the presented technique of calculating radius and coordinates of circular section center, the values of radii and coordinates of center point and distortions of Tank 9 at Forcados Terminal were determined at three oil levels from three epochs of observations using MATLAB program. The results are presented in tables 2 and 3 below. *Tables 2 – determination of radius of tanks and coordinates of their center* 

For	tank 9									
Tank Stu			Tank № 9 One cross section at level 2.0 m from the tank base							
Number										
sections of			oservations	were done v	vhen th	ne tank was	full of oil	level)		
	of monitor									
*	circular cr	oss 16 points	s covering the whole perimeter of tank cross section							
section			1							
<b>D</b> :	1 6 . 1		Q 1			_	T1.:1	1 6 . 1		
First cycle of observations at low oil level (3m)			Second cycle of observations low oil level (3m)			Third cycle of observations low oil level (3m)				
15.02.200				24.08.2004			10w oil level (3m) 7.10.2008			
15.02.200	55		24.00.200				/.10.2000			
	r, m	38.160		r, m	38.1	69		r, m	38.167	
Radius	$\sigma_r$ , mm	4.7	Radius	$\sigma_r$ , mm	3.6	0)	Radius	$\sigma_r$ , mm	3.6	
of		324772.44	of				of			
0	X, m	1	0	X, m	3247	72.440	0	X, m	324772.528	
		6.3			10			σ <sub>X</sub> ,	2.6	
s	$\sigma_X$ , mm		ŝ	σ <sub>X</sub> , mm	4.8		s	mm	3.6	
Coordinates center	Y, m	148129.01	Coordinates center	Y, m	1/181	29.028	Coordinates center	Y, m	148128.996	
din sr	1,111	3	adin Br	т, ш	1401	27.020	din er	1, 11	140120.770	
Coordi center	σ <sub>Y</sub> , mm	6.7	Coordi center	σ <sub>Y</sub> , mm	5.1		Coordi center	σ <sub>Y</sub> ,	4.8	
U S	0y, iiiii	0.7	03	0 <sub>1</sub> , iiii	0.1		03	mm	1.0	
	Diameter	76.320m			76.3	38m			76.334m	
		13724.226							13729.262m	
Actual m <sup>3</sup> Volume 118693.9t		m <sup>3</sup>			13730.700m <sup>3</sup>				3	
		118693.9b				49.8bbl			118737.4bb	
		bl							1	
		13681.102			13681.102 m <sup>3</sup>				13681.102	
	Nominal	$m^3$							$m^3$	
	Volume	118320.8b bl			1183	320.8bbl			118320.8bb	
	[	UI			1			1		
First evel	e of observat	ions	Second cy	cle of obser	vation	s	Third cyc	le of obse	rvations	
	evel (10m)		Second cycle of observation mid oil level (10m)			~	mid oil level (10m)			
15.02.200			24.08.200			7.10.2008				
r, m	38.213	Dedine	r, m	38.197		Radius	r, m	38.2	200	
σ <sub>r</sub> , mm	4.0	Radius	σ <sub>r</sub> , mm	3.1		Kaulus	σ <sub>r</sub> , mm	2.7		
X, m	324772.4	J3 Jo	X, m	324772 4	39 <sup>5</sup>				772.524	
	6					_			112.321	
$\sigma_X$ , mm	5.4	es	$\sigma_X$ , mm	<b>σ</b> <sub>X</sub> , mm 4.2		es	$\sigma_X$ , mm	3.6		
Y, m	148129.0	nat nat	<b>Y, m</b> 148129.00		007	nat	Y, m	148	148128.998	
	1	rdi ter	-			ordi ter	-			
$\sigma_{\rm Y},$ mm	5.7	Coordinates center	σ <sub>Y</sub> , mm	4.5		Coordinates center	$\sigma_{\rm Y},$ mm	<b>σ<sub>Y</sub>, mm</b> 3.9		
				76 204						
Diameter	r 76.426m			76.394m				76.4	00m	

Actual Volume	45874.584 m <sup>3</sup> 396745.8b bl			45836.176m <sup>3</sup> 396413.7bbl			45843.377m <sup>3</sup> 396476.0bbl								
Nominal Volume	45603.673 m <sup>3</sup> 394402.9b bl			45603.673m <sup>3</sup> 394402.9bbl		-	45603.673m <sup>3</sup> 394402.9bbl								
First cycle of full oil level 15.02.2003	of observations l (19m)		Second cy full oil lev 24.08.200	· · ·	ns		cle of observations evel (19m) 8								
r, m	38.220	D 11	r, m	38.230	D 1'	r, m	38.234								
σ <sub>r</sub> , mm	3.3	Radius	σ <sub>r</sub> , mm	3.7	— Radius	σ <sub>r</sub> , mn	n 2.2								
X, m	324772.436	<sup>1</sup>	X, m	324772.434	10	X, m	324772.5208								
$\sigma_{\rm X}$ , mm	4.4	ates	$\sigma_{\rm X}$ , mm	4.9	ate:	$\sigma_X$ , mr	<b>n</b> 3.0								
Y, m	148129.013	dina	dina	dina	dina	dina	ding	dina	dina	dina	Y, m	148129.018	dina	Y, m	148128.999
σ <sub>Y</sub> , mm	4.4	Coordinates of center	σ <sub>Y</sub> , mm	5.2	Coordinates of center	σ <sub>Y</sub> , mr	<b>m</b> 3.2								
Diameter	76.44m			76.460m			76.468m								
Actual Volume	87193.646 m <sup>3</sup> 754093.4bb l			87239.279m <sup>3</sup> 754488bbl			87257.536m <sup>3</sup> 754645.9bbl								
Nominal Volume	86646.979 m <sup>3</sup> 749365.5bb l			86646.979m <sup>3</sup> 749365.5bbl			86646.979m <sup>3</sup> 749365.5bbl								

Table 3 - Tank 9 Distortions

STUDS	2003Low	2003Mid	2003Full	2004Low	2004Mid	2004Full	2008Low	2008Mid	2008Full
	mm	mm	mm	mm	mm	mm	mm	mm	mm
STUD1	-87.492	-56.672	-23.244	438.05	-13.067	458.06	-57.933	-25.136	9.6489
STUD9	-118.96	-81.888	-55.86	-4.9426	-43.617	-4.1533	-49.708	-24.706	6.9
STUD16	-74.91	-54.681	-13.414	-12.1	-10.352	14.263	-91.859	-9.7979	28.24
STUD8	-102.91	-74.732	-39.184	-6.7749	-33.592	-8.3302	-80.34	-45.213	0.8752
STUD2	-81.624	-47.132	-31.219	18.099	13.479	47.923	-59.957	-30.896	3.1295
STUD10	-119.93	-101.71	-82.46	-31.868	-66.918	-27.153	-47.711	-21.012	16.677
STUD4	-80.7	-27.692	7.9384	-15.906	-17.04	15.722	-71.06	-46.995	-2.153
STUD12	-167.47	-126	-79.96	-64.336	-108.38	-69.696	-67.424	-38.07	-2.1742
STUD3	-65.995	5.4712	24.493	23.594	12.181	49.431	-78.773	-46.509	-14.287
STUD11	-98.772	-108.43	-57.164	-32.082	-58.302	-21.456	-26.554	-1.742	33.226
STUD5	-65.727	-15.258	-0.6684	-35.492	-52.224	-22.073	-57.058	-33.602	7.2398
STUD13	-114.73	-91.798	-68.178	60.313	26.714	52.766	-59.831	-41.05	-6.615
STUD7	-21.403	0.60187	28.374	74.753	30.868	75.713	-10.013	8.9676	58.307
STUD15	-93.652	-63.679	-37.232	-8.4607	23.627	46.115	-45.482	-24.654	21.3
STUD6	-41.42	0.39721	20.854	4.2939	9.8724	55.221	-45.5	-11.735	36.221
STUD14	-70.983	-51.525	-24.8	65.465	54.179	80.974	-29.982	-10.558	38.071

The tables above are the results of the diameter of the tank and it accuracy, coordinates of the centre, and the actual volume of oil in the tank at different oil level and at three epochs of observations.

At 3m oil level in year 2003, the diameter was found to be 76.320m, while the tank nominal diameter was given as 76.20m. The crude oil volume was found to be 11869.9bbl, while the nominal volume is 118320.8bbl; this in excess of 373.1bbl, distortion was maximum at stud9 with value of 118.96mm and minimum at stud7 with a value of 21.403mm. In 2004, diameter was found to be 76.338m; oil volume 118749.8bbl, again in excess of 429bbl, distortion was maximum at stud1 with value of 438.05mm and minimum at stud1 with value of 4.94mm. In 2008, the diameter was found to be 76.334m, actual oil volume was found to be 118737.4bbl, an oil excess of 416.6bbl, distortion was maximum at stud16 with value of 91.86mm and minimum at stud7 with a value of 10.01mm

This is an indication that there is an increase in diameter of the tank from the nominal diameter (76.2m) to 76.320 in 2003, 76.338 in 2004 and 76.334 in 2008. Also there was a corresponding increase in oil level from epoch to epoch.

When the oil volume was increased from 3m to 10m for the three epochs, the following was also deduced. In 2003, diameter was 76.426m, excess oil volume was 2342.9bbl, and distortion was maximum at stud11 with value of 108.43mm and minimum at stud6 with a value of 0.40mm. In 2004, diameter was 76.44m and excess oil volume was found to be 2010.8bbl, distortion was maximum at stud12 with value of 108.38mm and minimum at stud6 with a value of 9.87mm. In 2008, the tank diameter was found to be 2073bbl; distortion was maximum at stud4 with value of 46.99mm and minimum at stud11 with value of 1.74mm

Again, the oil volume was increased from 10m to 19m for the three epochs, the following was also deduced. In 2003, diameter was 76.44m, excess oil volume was 4727.9bbl, and distortion was maximum at stud10 with value of 82.46mm and minimum at stud5 with a value of 0.67mm. In 2004, diameter was 76.46m and excess oil volume was found to be 5122.5bbl, distortion was maximum at stud1 with value of 458.06mm and minimum at stud9 with a value of 4.15mm In 2008, the tank diameter was found to be 5280.4bbl, distortion was maximum at stud16 with value of 28.24mm and minimum at stud8 with a value of 0.88mm

The diameter was computed from the radius and compared with the nominal diameter at 19m oil level. For 2003 epoch, there is an expansion of 0.24m, in 2004 the expansion was found to be 0.26m and for 2008 the expansion was found to be 0.268m. From the above result, i.e. the determined diameter, actual oil

volume and coordinates of center of tank no. 9, it can be seen that there are a clear difference in these values between each epoch of observations and the nominal volume and diameter. This we mean that there has been deformation in the wall of the tank since after their construction.

#### CONCLUSIONS

Monitoring of tanks and tanks wall helps in identifying and quantifying deteriorations which may lead to tank failure. The history of tank disaster throughout the world reveals that problems often arise undetected due to inaccurate evaluation of the tank defects.

For an effective tank monitoring programme, the equipment used for the monitoring must be precise and of the highest quality. The monitoring personnel must be experienced in not only data capture but also the analysis of the acquired data. The period of observation should be every year and consistent throughout the life of the tank

Further studies should be carried out on the tank to ascertain the character of the tank over the years. The use of the mathematical model and associated designed MATLAB program to determine the radius and coordinates of center of circular oil tanks from geodetic data especially during the process of monitoring the structural deformation was found to be very correct and economical. The period of observation should be every year and consistent throughout the life of the tank. The results obtained in this study may however be acceptable to the structural Engineer depending on the tank specifications and its properties at the design stage.

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